

# Retrieved from DalSpace, the institutional repository of Dalhousie University

http://hdl.handle.net/10222/81975

Version: Postprint

Publisher's version: Choi. H., Lake, C.B. and Hills, C.D. 2020. Examining particle breakage for a manufactured aggregate from cement kiln dust. ASCE Journal of Hazardous, Toxic, and Radioactive Waste, 24(1): 04019029. DOI: 10.1061/(ASCE)HZ.2153-5515.0000468

1	Particle Size Effects on Breakage of ACT Aggregates Under Physical and							
2	Environmental Loadings							
3	Hun Choi <sup>1</sup> , Craig B. Lake <sup>2</sup> , and Colin D. Hills <sup>3</sup>							
4 5 7 8 9 10 11 12 13								
14	<sup>1</sup> Former Graduate Student, Civil and Resource Engineering Department, Dalhousie							
15	University, Canada.							
16	<sup>2</sup> Professor, Civil and Resource Engineering Department, Dalhousie University, Canada.							
17	<sup>3</sup> Professor, Centre for Contaminated Land Remediation, University of Greenwich,							
18	United Kingdom; Adjunct Professor, Civil and Resource Engineering Department,							
19	Dalhousie University, Canada							
20								
21								
22	*Corresponding author: Craig B. Lake, Civil and Resource Engineering Department,							
23	Dalhousie University, Halifax, Nova Scotia, Canada							
24	E-mail: craig.lake@dal.ca, Tel.: +1 (902) 494 3220 Fax: +1 (902) 494 3108							
25								

#### 26 Abstract

27 Aggregates manufactured from fine-grained thermal waste residues using accelerated carbonation technology (ACT) represent a potential sustainable alternative to natural 28 29 aggregates. However, for these manufactured products to compete with virgin stone in 30 geotechnical applications, their durability under mechanical and environmental 31 loadings must be assessed. This paper describes particle breakage that occurs for 32 different grain sizes (entire sample, 5mm-2.5mm, and 2.5mm-1.25 mm) of a cement 33 kiln dust accelerated carbonated manufactured aggregate after undergoing triaxial 34 compression, triaxial shear and freeze/thaw (f/t) testing. It is shown that the particle 35 breakage of the aggregate is dominated by the larger (5mm-2.5mm) size fraction of the 36 sample under all loading conditions. Particle breakage results from f/t testing showed that the 5mm-2.5mm size corresponded to similar or slightly less particle breakage than 37 that under triaxial shear, while the particle breakage of the 2.5mm-1.25mm aggregate 38 39 after 20 cycles of freeze-thaw was relatively small. The performance of the carbonated 40 aggregate in terms of relative breakage was similar or slightly better than natural calcareous sand results in the literature. 41

42 Keywords: manufactured aggregate, particle breakage, accelerated carbonation,43 compression, shear, freeze/thaw

44

2

#### 45 Introduction

Sustainable aggregate manufacturing represents a potential solution to limited 46 47 aggregate supply in some geographical regions, and a sustainable waste management solution for selected high-volume wastes currently landfilled. Challenging regulatory 48 49 approvals required for environmental permitting in developed countries also suggests that manufactured aggregates will become increasingly important in the future. The use 50 51 of accelerated carbonation technology (ACT) has been shown to manufacture light-52 weight aggregates from fine-grained thermal waste residuals while at the same time 53 sequestering  $CO_2$  during the aggregate production process (Gunning et al. 2009). This 54 sustainability aspect of the aggregate has led to the development of ACT manufacturing 55 plants in the United Kingdom. Various researchers (e.g. Fernández et al. 2004; Domingo 56 et al. 2006; Costa et al. 2007) have described the science behind the ACT process and 57 it hence will not be repeated here.

58 Current application of ACT manufactured aggregates in the United Kingdom 59 is replacement aggregates for concrete blocks (Gunning et al. 2011). For these 60 aggregates to be used in broader applications such as engineered fill for roadway 61 construction, their sustained durability under mechanical and environmental loadings 62 is critical. The durability of coarse-grained soil particles in geotechnical applications 63 has been the subject of numerous studies in the literature due to concerns related to 64 particle breakage (i.e. large dams (Marsal 1967), deep foundations (Klotz & Coop 65 2001), roadways (Zheghal 2009) and petroleum applications (Zheng & Tannant 2016). 66 Work by Marsal (1967); Lee & Farhoomand (1967); Hardin (1985) represent examples 67 of early studies performed to investigate the durability of soil particles by examining 68 particle breakage. It has been shown in these, and other studies, that the amount of soil particle breakage will depend, inter alia, on the individual soil particle's mineralogy,
shape, and size (Lee & Farhoomand 1967; Hardin 1985). Stress level, stress path, and
time effects (Coop 1990; Lade et al. 1996; Yamamuro & Lade 1993 Altuhafi & Coop
2011) also play key roles in particle breakage.

73 Reported ACT manufactured aggregate individual particle strengths are in the 74 order of 0.5 MPa to 1 MPa (Lake et al. 2016) and hence are potentially susceptible to 75 particle breakage, similar to weaker grain soil particles such as calcareous sands (e.g. 76 Coop 1990). As accelerated carbonated aggregates are commercially available and 77 being further developed, data on aggregate breakage under different loadings is needed 78 to elucidate potential applications outside of bound systems. Thus, the purpose of this 79 paper is to provide a relative comparison of particle breakage for an accelerated carbonated aggregate developed from cement kiln dust previously described by Lake et 80 81 al. (2016). In this paper the manufactured aggregate is subjected to a series of triaxial 82 tests (drained isotropic compression and drained isotropic compression followed by 83 drained axial shear) as well as (f/t) tests. The influence of aggregate size/gradation on 84 the particle breakage is evaluated by examining changes in grain size distributions of 85 the samples by calculating relative breakage, as defined by Hardin (1985). Given that the ACT manufacturing process can control the final size of the aggregate, 86 87 understanding the role of size in particle breakage is useful as a potential tool for 88 improving the durability performance of the product.

89

#### 90 Materials and Methods

91 Cement Kiln Dust ACT Aggregate Production Process

92 The testing performed in this paper used a cement kiln dust (CKD) material previously

described by Lake et al. (2016). In summary, the material was obtained from a Lafarge
cement plant in Brookfield (Nova Scotia, Canada) in August 2014. The CKD consists
of calcium, iron, silica, aluminum, magnesium and potassium oxides, with 42% CaO
content being the key carbonate-able mineral in producing the aggregate.

97 The pelleting process for carbonated aggregate manufacturing described by 98 Lake et al. (2016) was modified slightly in an attempt to produce a stronger aggregate 99 than the  $\sim 1$ MPa strength from that work. Pelletizing involved pre-mixing 400g (dry 100 mass) of CKD with 130g of water using a laboratory paddle mixer. During this phase 101 of the mixing process, CKD with water was mixed for 45 to 60 seconds at a 50 rpm 102 mixing speed followed by 30 to 60 seconds at 120 rpm. Subsequently, an additional 60g 103 of CKD was added at a 120 rpm mixing speed and then 40g CKD was added to mixer; 104 then mixed for 30 seconds at 50 rpm to produce rounded aggregates. The mix was then 105 placed in a rotating drum with CO<sub>2</sub> saturation for 15 minutes at 50 rpm to complete the 106 process. The carbonated aggregates in this study were air-cured at  $20 \pm 2^{\circ}$ C and  $\sim 25\%$ 107 relative humidity conditions in a lab for at least 28 days prior to being subjected to any 108 testing. As reported by Lake et al. (2016), the relative density (i.e. specific gravity) of 109 the aggregates for this process is 2.1. When placed in a loose compaction state, the bulk 110 density is  $1200 \text{ kg/m}^3$ .

In this paper, three different particle size distributions were assessed for particle breakage. The entire particle size distribution produced from the ACT process, referred to as "EPS" in this paper, was used initially for testing. Additional aggregate was prepared using the same techniques for the EPS but after curing, the aggregate was segregated into two distinct size fractions. This was accomplished by passing the material through the 5 mm sieve, followed by the 2.5 mm sieve, followed by the 1.25 mm sieve. Material retained on the 5 mm sieve and passing the 1.25 mm sieve was discarded. Material retained on the 2.5 mm sieve is referred to as the "5-2.5PS" sample in this paper and the material retained on the 1.25 mm sieve is referred to as the "2.5-1.25PS" sample.

121

## 122 Aggregate Testing

## 123 <u>Single Aggregate Pellet Strength</u>

To provide an initial assessment of individual particle strength, a single pellet compressive strength test (ASTM D4179-11, 2011), as described by Lake et al. (2016) was used. Ten (10) samples were taken from the two carbonated aggregate grain sizes prepared (i.e. 5-2.5PS; 2.5-1.25PS) and subjected to single pellet compressive strength tests. The mean value of the strength was reported. Particle strengths were measured at 7, 15 and 27 days of air-curing to allow comparison to aggregates presented by Lake et al. (2016).

131

#### 132 Drained Isotropic Triaxial Compression Tests

133 To assess the extent of particle breakage of the accelerated carbonated aggregate during 134 drained isotropic triaxial compression testing, the three grain sizes were subjected to 135 isotropic compression tests similar to that described by Lake et al. (2016). For the EPS 136 material, two subsamples were taken of the prepared aggregate for each test; one was 137 submitted to a grain size analysis and the second used in the triaxial compression tests. 138 For the compression tests, the given aggregate size was lightly compacted in a split 139 mould (70mm inside diameter x 150mm high) and subjected to saturation under 35 kPa 140 effective confining pressure in a triaxial cell. The sample was then subjected to

141 consolidation under the desired effective confining pressure (600, 800, 1000, 1200 or 142 1400 kPa) to produce the isotropic compression conditions desired. The 1400 kPa 143 represents the limit of the cell pressure available. The test progressed until the specimen 144 reached equilibrium in a drained state at the desired effective consolidation stress. After 145 this was achieved, the grain size curve (by dry weight) of the sample was determined 146 in order to assess the extent of particle breakage that occurred. For the 5-2.5PS and 2.5-147 1.25PS material, only the grain size distributions after the compression tests were 148 performed. For all tests, the grain size distributions of samples before and after triaxial 149 testing were plotted and compared visually as well as calculating the relative breakage 150 of the particle using the method developed by Hardin (1985). Hardin (1985) assumed 151 that the breakage of particles terminated when the gradation curve of a soil reached a 152 stable condition (i.e. when the final condition achieved was when all particles were 153 smaller than 0.074 mm (sieve No. 200)). The relative breakage (Br) of soil can then be 154 calculated from the following equation (Hardin, 1985):

155 
$$B_r = \frac{B_t}{B_p}$$
(1)

156 Where B<sub>p</sub> is the breakage potential (calculated as the area between the original grain 157 size distribution curve and a vertical line drawn at the 0.074mm sieve size) and  $B_t$  is 158 the total breakage (calculated as the area between the initial and final grain size 159 distribution curves). A value of B<sub>r</sub> of zero would mean there was no change in grain 160 size distribution (i.e. no breakage) during testing while a value of one would mean 161 that all particles (i.e. maximum breakage) were reduced to sizes less than 0.074 mm 162 size during testing. Readers can access good visual descriptions of the value Br from 163 Hardin (1985).

#### 164 Drained Isotropic Compression, Drained Axial Shear Triaxial Tests

165 To assess the extent of particle breakage of the carbonated aggregate under shearing 166 conditions, the three different particle size fractions (i.e. EPS, 5-2.5PS, 2.5-1.25PS) 167 were subjected to drained isotropic compression followed by drained axial shear. 168 Samples were prepared similarly to those described in the previous section. In summary, 169 the given aggregate size was lightly compacted in a split-mold and subjected to 170 saturation under an effective confining pressure of 35 kPa (ASTM 4767-11, 2011). The 171 sample was then subjected to consolidation under the desired effective confining 172 pressure (600, 800, 1000, 1200 or 1400 kPa) and then subsequently sheared under 173 drained conditions at an axial displacement rate of 1.5 mm/min. Axial loading was 174 terminated at 15% axial strain to provide a common strain level. Similar to the triaxial 175 compression tests described in the previous section, grain size distributions were 176 compared before and after testing and the relative breakage calculated.

177

## 178 <u>Freeze/Thaw Durability Testing</u>

179 F/t durability testing, similar to that described by Lake et al. (2016) was performed on 180 the three different carbonated aggregate sizes (i.e. EPS, 5-2.5PS, 2.5-1.25PS). To 181 summarize, the aggregate samples were soaked in water for 4 hours and then surface 182 dried for approximately 15min. The test aggregate samples were then subjected to 10 183 or 20 f/t cycles that included freezing to  $-17.5 \pm 2.5$  °C for 24h and then thawing in a 184 water bath at room temperature ( $\sim 20 \pm 2^{\circ}$ C) for 4h (BSI BS EN 13055-4, 2016). The 185 20 cycles is recommended in the standard referenced above while 10 cycles allowed an 186 examination of breakage at an intermediate step in the f/t process. At the end of the f/t187 cycles, the samples were subjected to grain size analyses to compare particle size

distributions before and after the f/t cycling. For comparison to triaxial compression
and/or triaxial shear testing, relative breakage was calculated at the end of each test.

190

## 191 Results

## 192 Single Aggregate Pellet Strength

Particle strengths for the two sizes increased as the curing time increased. The 5-2.5PS aggregate achieved strengths of 1.8 MPa, 2.6 MPa, and 2.9 MPa after 7, 15, and 27 days of curing, respectively. The 2.5-1.25PS aggregate achieved strengths of 2.0 MPa, 2.8 MPa and 3.0 MPa after 7, 15, and 27 days of curing respectively. For these simple particle strength tests, there appears to be little, if any, difference in particle strength between the two particle sizes tested. These test results confirmed that the majority of the aggregate curing was completed after 28 days.

200

## 201 Drained Isotropic Triaxial Compression Testing

#### 202 <u>EPS</u>

203 Figure 1 shows the particle size distributions for the EPS size of accelerated carbonated 204 aggregate before and after the drained isotropic triaxial compression tests for the five 205 effective confining pressures employed (600, 800, 1000, 1200, and 1400 kPa). As 206 seen from Figure 1, the grain size distributions of the sample visually changed little 207 during these tests. At the higher confining pressures (1200 kPa and 1400 kPa), the grain 208 size did change slightly as is evident by the increase in the finer fraction of the soil. The 209 resulting relative breakage (B<sub>r</sub>) values for the EPS size of ACT aggregate is shown on 210 the inset of Figure 1. Similar to the visual observations noted above,  $B_r$  increased as the 211 confining pressure increased; the greatest increase occurring from 1000 to 1200 kPa.

#### 212 <u>5-2.5PS</u>

213 Figure 2 shows the particle size distributions for the ACT aggregate before and after 214 the drained isotropic triaxial compression testing for the 5-2.5PS material. The initial 215 particle size distribution is a straight line as there is only one particle size present in 216 this sample. As seen from Figure 2, the grain size distributions of the sample visually 217 changed more than that for the EPS during these tests. It appears when examining 218 these grain size plots that there is a more pronounced effect on particle crushing 219 compared to that of the entire grain size distribution and that as the confining pressure 220 increased, the breakage also appeared to increase (i.e. appearance of more finer grain 221 fraction in the sample). The resulting relative breakage for this size is shown on the 222 inset of Figure 2, and confirms this observation. It is also noted that the relative 223 breakage found for this aggregate size was higher than that of the EPS. This 224 observation of how larger particle size, and grain size uniformity can result in 225 increases in particle breakage is consistent with Cassini et al. (2013) and Altuhafi & 226 Coop (2011).

227

## 228 <u>2.5-1.25PS</u>

Figure 3 shows the particle size distributions for the accelerated carbonated aggregate before and after the drained isotropic triaxial compression testing for the 2.5-1.25PS material. As with the 5-2.5PS material, the initial particle size distribution is a straight line as only one particle size is present in this sample. As seen from Figure 3, the grain size distributions of the sample visually changed less than the 5-2.5PS sample and that of the EPS during these tests. It also appears that there was some minor particle breakage as the confining pressure increased. The resulting relative breakage for this size as shown on the inset of Figure 3 confirms this observation. It is also noted that the
breakage parameters found for this size aggregate were lower than that of the EPS and
the 5mm-2.5mm sample. This is perhaps not surprising when comparing to the EPS as
it is likely most of the particle breakage occurred in the 5mm-2.5mm fraction of the
sample.

241

## 242 Drained Isotropic Compression, Drained Axial Shear Triaxial Test

243 <u>EPS</u>

244 Figure 4 shows the particle size distributions for the carbonated aggregate before and 245 after the drained triaxial shear testing for the EPS particle size distribution for the five 246 effective confining pressures employed (600, 800, 1000, 1200, and 1400 kPa). As 247 seen from Figure 4, compared to the isotropic triaxial compression tests for the EPS, 248 there was noticeably more particle breakage from the shear testing. There was also more 249 particle breakage as the confining pressure was increased. The resulting relative 250 breakages for the carbonated aggregates are shown on the inset of Figure 4. Similar to 251 the visual observations noted above, the relative breakage increased as the confining 252 pressure increased. Also to note is the higher values of Br relative to the isotropic triaxial 253 compression tests reported in Figure 1. As will be discussed later, this is due to the 254 higher mean stress in the samples relative to isotropic triaxial compression tests and the 255 shear being generated in the samples.

256

## 257 <u>5-2.5PS</u>

Figure 5 shows the particle size distributions for the carbonated aggregate before and

259 after the drained triaxial shear testing for the 5-2.5PS material. As seen from Figure 5, 260 the grain size distributions of the sample visually changed more than that for the EPS 261 during these tests, similar to that observed for the isotropic triaxial compression tests. 262 It also appears when examining these grain size plots that there is a more pronounced 263 effect on particle crushing compared to that of the EPS and, that as the confining 264 pressure increased, there was the appearance of more of a finer grain fraction. The 265 resulting relative breakage for the carbonated aggregate, as shown on the inset of 266 Figure 5, confirms this observation. It is also noted that the  $B_r$  values found for this size 267 aggregate were higher than that of the EPS and that the values of  $B_r$  were higher than 268 that for the isotropic triaxial compression tests.

269

#### 270 <u>2.5-1.25PS</u>

271 Figure 6 shows the particle size distributions for the 2.5-1.25PS carbonated aggregate 272 before and after the drained triaxial shear testing. As seen from Figure 6, the grain size 273 distributions of the samples for triaxial shear visually changed less than both the 5-274 2.5PS sample and that of the EPS during these tests. It does visually appear that there 275 was some minor particle breakage as the confining pressure increased, albeit less than 276 the other two sizes. The resulting relative breakage  $(B_r)$  for the 2.5-1.25PS material, as 277 shown on the inset of Figure 6, confirms this observation. It is also noted that the 278 particle breakage found for this size aggregate was lower than that of the EPS and the 279 5mm-2.5mm sample. Also noted is the relative increase in  $B_r$  compared to triaxial 280 compression testing.

281

## 282 Freeze/Thaw Cycle Effect on Particle Breakage

283 Figure 7 shows the grain size distribution of the EPS before and after f/t cycle testing. 284 After 10 cycles of f/t it is apparent that the grain size distribution of the EPS has a finer 285 grain size distribution and after 20 cycles of f/t, this particle size became even finer. 286 The percent passing the 2.5 mm sieve has increased the most in both instances, 287 indicating the larger portion of the sample may be exhibiting more breakage. This 288 phenomenon of particle breakage of the larger size aggregate can be examined further 289 when examining the isolated sizes. The 5-2.5PS material exhibited significantly more 290 breakage than that of 2.5-1.25PS. The 2.5-1.25PS grain size curve visually changed 291 little.

Although particle breakage is usually for mechanical loading applications, it is interesting to calculate  $B_r$  for the f/t cycle tests (see inset of Figure 7). The values of  $B_r$ corresponded with the visual observations of the grain size curves (more breakage with f/t cycles at 20 than 10; at a given f/t/ cycle, breakage increases 5-2.5PS>EPS>2.5-1.25PS).

297

298 Discussion

299 Particle Breakage Comparison between Triaxial Compression, Triaxial Shear, and
300 Freeze/Thaw Testing

It is useful to compare the particle breakage obtained from the triaxial compression, triaxial shear, and f/t cycle testing to ascertain the effect that different loading conditions would have on the particle breakage of the carbonated aggregate. Figure 8 shows the values of B<sub>r</sub> obtained from these three different test methods. For ease of comparison, results are presented in terms of mean stress,  $\rho' (\rho' = (\sigma'_1 + \sigma'_2 + \sigma'_3)/3)$  where  $\sigma'_1$  and  $\sigma'_3$  306  $(\sigma'_2 = \sigma'_3)$  for these triaxial tests) are the major principal effective stress (i.e. axial stress) 307 and minor principal stress (i.e. effective confining stress) at failure. Since triaxial 308 compression and triaxial shear tests were performed under the same effective confining 309 pressures, the use of mean stress,  $\rho'$ , allows the "additional" axial stress to be accounted 310 for when examining stresses. For the purpose of this paper, the mean stress at the end 311 of the 15% strain level (i.e. termination of the test) was used. It should also be noted 312 that the f/t tests were performed under "zero external mean stress" conditions and hence 313 the grey shading observed on Figure 8 represents the range of  $B_r$  values for the 10 and 314 20 f/t cycles for each particle size.

315

As previously discussed, it is apparent that for the various triaxial tests, the influence of shear resulted in the highest amount of particle breakage (i.e. when comparing similar mean stresses). As discussed by Coop (1990), the true measure of particle breakage is obtained from critical state conditions (which were not reached in these tests) and hence this must be remembered when examining these test results.

321 It is noted that for a given loading condition, the 5-2.5PS aggregate underwent more 322 relative breakage ( $B_r$ ) than that of the 2.5-1.25PS aggregate. When one examines the f/t 323 cycle testing results relative to the triaxial compression and triaxial shear results, it is 324 interesting to see that the particle breakage from f/t cycle testing for the EPS and 325 5mm-2.5mm size corresponds to much higher particle breakage than triaxial 326 compression (for similar particle sizes) testing but similar to slightly lower particle 327 breakage as that under triaxial shear. This suggests that environmental factors such as 328 f/t cycle may be just as important a design consideration for the ACT aggregate as that 329 compared to loading conditions. In contrast, the particle breakage of the 2.5-1.25PS

- aggregate after 20 cycles of f/t is relatively small and similar in magnitude to the same
- 331 size aggregate under triaxial compression test conditions.
- 332

## 333 Comparison of ACT Aggregate Particle Breakage to Other Studies

334 Even though there are many different approaches to assessing particle breakage in the 335 literature, it is useful to try, at least from a qualitative standpoint, to compare particle 336 breakage obtained from this study to that of previous studies. For the purposes of this 337 study, calcareous sands studied by Shipton & Coop (2012) (using data from 338 Coop (1990)) and Shahnazari & Rezvani (2013) as well as predominantly silica sands 339 tested by Mun & McCartney (2017) are used as reference materials. The silica sand 340 tested by Mun & McCartney (2017) is relatively durable compared to the calcareous 341 sands tested by Coop and his coworkers. These two types of materials represent natural 342 aggregates, both used in construction applications.

343

For this research, some values for  $B_r$  for other studies were estimated visually from plots and hence this may lead to some discrepancy. However, from a qualitative perspective, approximations were deemed sufficient. Also to note is that the results of Shipton & Coop (2012) and some of Shahnazari & Rezvani (2013) were a mix of isotropic compression tests and drained triaxial shear tests. As well, the tests by Shipton & Coop (2012) were run to higher strain levels than this study (i.e. would account for more particle breakage).

Figure 9 shows  $B_r$  values for the various studies. Also shown on Figure 9 are the results for the 5-2.5PS aggregate and the 2.5-1.25PS aggregate in this study. The results of the EPS in this study always fell between these two samples and hence are 354 not included on Figure 9 for clarity purposes. As shown on Figure 9, at a given B<sub>r</sub>, it 355 took significantly more mean stress for Mun and McCartney (2017) to achieve the same 356 particle breakage as compared to the ACT aggregate in this study. This is not surprising 357 given the relative differences in mineralogy and individual particle strengths of the 358 aggregates. If one compares the ACT manufactured aggregate to the natural calcareous 359 sand results for similar values of Br it can be seen that similar or less mean stress is 360 required to achieve a similar particle breakage as that in this study. This is somewhat 361 encouraging in that calcareous sands have shown to be adequate materials for 362 construction projects in geotechnical applications provided loading and/or strain level 363 is limited to control particle breakage.

364

## 365 *Implication for Qualitative Manufacturing of ACT Aggregates*

366 There are several practical implications from this work related to ACT aggregate 367 manufacturing. Firstly, it is apparent that similar to other particle breakage studies in 368 the literature, larger particle sizes (i.e. 5-2.5PS size fraction in this study) are more 369 susceptible to particle breakage relative to smaller size particles (2.5-1.25PS). This 370 suggests that if aggregates manufactured by ACT as in this study were to be used in 371 geotechnical applications in which they will be subjected to shear and/or compression 372 conditions where the mean stresses approached or exceeded those in this research, 373 aggregates in the 2.5-1.25PS size range would perform better than those in the 5 mm-374 2.5mm range. Given that the particle sizes can be controlled in the commercial 375 aggregate manufacturing process, a move to smaller particles may be preferred in 376 geotechnical applications, especially those in shear applications (i.e. roadways, 377 embankment slopes). If the aggregate is to be used in applications where shear is limited

and loading is predominately isotropic compression, then the amount of particle breakage is expected to be small (e.g. wide fills). For example, for the 2.5-1.25PS size, B<sub>r</sub> values were less than 0.01 for mean stresses up to 1400 kPa in triaxial compression tests. This stress range would be above most common stress applications for urban developments.

383

#### 384 *Exposure to Freeze/Thaw Cycling*

385 As shown in Figure 8, for f/t cycle tests carried out to only 20 cycles, resultant relative 386 breakage was at or near that from the triaxial shear tests for the 2.5-1.25PS carbonated 387 aggregate. This suggests that care should be taken to ensure exposure to f/t cycles is 388 limited for this size of aggregate. However, the f/t performance of the 2.5-1.25PS size 389 was significantly better than that of the 5-2.5PS size. For the 20 f/t cycles, limited 390 breakage was observed. This observation should be taken within the context that 20 f/t 391 cycles may not be representative of field applications (i.e. more f/t cycles may be 392 present in the field). This is an area in which further testing would be useful. The 393 manufacturing of carbonated aggregates smaller than 2.5mm appears to be desirable if 394 f/t is a concern.

395

## **Summary and Conclusions**

This paper has presented the results of various tests designed to examine the amount of particle breakage that occurs for the CKD-derived accelerated carbonated aggregate developed in this study. Of particular emphasis in this study is the role of particle size on relative breakage ( $B_r$ ) for the aggregate product. Triaxial compression, triaxial shear and f/t cycling tests were performed on the carbonated aggregate using the entire grain size distribution and with isolated particle sizes (5-2.5PS and 2.5-1.25PS) to assess particle breakage. The grain sizes before and after testing were used to calculate the relative breakage,  $B_r$ . This parameter provides a fairly simple technique to provide relative comparisons between grain sizes and also allows comparisons to be made with previous studies related to calcareous sands and other more durable sands.

It was shown in this study that similar to other studies in the literature for natural soils, the majority of particle breakage in the accelerated carbonated aggregate occurred in the large particle size (5-2.5PS) relative to the smaller particle size (2.5-1.25PS). This observation was present regardless of the test method performed.

411 Particle breakage results from f/t testing showed that the EPS and 5-2.5PS 412 materials corresponded to similar or slightly less particle breakage as that under triaxial 413 shear. This suggests that environmental factors such as f/t may be as important a 414 consideration in application to loading conditions. In contrast, the particle breakage of 415 the 2.5-1.25PS material after 20 cycles of f/t was small and similar in magnitude to the 416 same size aggregate under low to medium confining pressures for triaxial compression. 417 The performance of the accelerated carbonated aggregate in terms of relative breakage 418 (i.e. B<sub>r</sub>) was similar or slightly better than natural calcareous sand results found in the 419 literature but substantially higher than more durable mineral fraction sands. This is an 420 interesting finding in that calcareous sands, although not the most desirable sand to use 421 in construction, can be used when its geotechnical limits are known and loads and 422 strains are limited to account for this performance. More work is required to examine 423 these limited conditions for the accelerated carbonated aggregate, especially at higher 424 strain levels and higher levels of f/t performance.

425

4	126				
4	127				
4	128				
4	129				
4	130				
4	131				
4	132				
4	433				
4	134				
4	135				
4	136				
4	137				
4	138				
4	139				
4	140				
	141				
4	142				
4	143				
4	144				

## 445 Figure Captions

446

- Fig. 1. Particle size distributions for the EPS ACT aggregate sample before and aftertriaxial compression.
- 449 Fig. 2. Particle size distributions for the 5-2.5PS ACT aggregate sample before and after
- 450 triaxial compression.
- 451 Fig. 3. Particle size distributions for the 2.5-1.25PS ACT aggregate sample before and
- 452 after triaxial compression.
- Fig. 4. Particle size distributions for the entire size ACT aggregate sample before and
- 454 after triaxial shear.
- 455 Fig. 5. Particle size distributions for the 5-2.5PS ACT aggregate sample before and after456 triaxial shear.
- 457 Fig. 6. Particle size distributions for the 2.5-1.25PS ACT aggregate sample before and458 after triaxial shear.
- Fig. 7. ACT aggregate particle size distributions, before and after 10 and 20 cycles offreeze/thaw.
- 461 Fig. 8. Relative breakage ( $B_r$ ) vs Mean Stress ( $\rho'$ ).
- 462 Fig. 9. Comparison of  $B_r$  values in this study to literature values.
- 463
- 464

#### 465 **References**

- Altuhafi, F. N., and Coop, M. R. (2011) "Changes to particle characteristics associated
  with the compression of sands." *Geotechnique*, 61(6), 459–471. DOI
  10.1680/geot.9.P.114.
- ASTM International. (2011). "Standard test method for single pellet crush strength of
  formed catalysts and catalyst carriers." *ASTM D 4179-11*., West Conshohocken, PA,
  USA.
- 472 ASTM International (2011). "Standard test method for consolidated drained triaxial
- 473 compression test for soils." *ASTM D4767-11*., West Conshohocken, PA, USA.
- BSI (British Standards Institution). (2016). "Lightweight aggregates." BSI BS EN *13055-4.*, London, UK. Carbon8. <a href="http://c8a.co.uk/>(Mar. 07, 2018).">http://c8a.co.uk/>(Mar. 07, 2018).</a>
- Cassini, F., Viggiani, G.M.B., and Springman, S.M. (2013) "Breakage of an artificial
  crushable material under loading." *Granular Matter*, **15**(5), 661-673 DOI
  10.1007/s10035-013-0432-x.
- 479 Coop, M. (1990). "The mechanics of uncemented carbonate sands." *Geotechnique*, 40
  480 (4), 607–615.
- 481 Costa, I., Baciocchi, R., Polettini, A., Pomi, R., Hills, C.D., and Carey, P.J. (2007).
- 482 "Current status and perspectives of accelerated carbonation processes on municipal
- 483 waste combustion residues." *Environmental Monitoring and Assessment*, **135**, 55-75.
- 484 Domingo. C., Loste, E., Gomez-Morales, J., Garcia-Carmona, J., and Fraile, J. (2006).
- 485 "Calcite precipitation by a high-pressure CO<sub>2</sub> carbonation route." Journal of

486 *Supercritical Fluids*, **36**(3), 202–215.

- Fernández, B. M., Simons, S. J. R., Hills, C. D., and Carey, P. J. (2004). "A review of
  accelerated carbonation technology in the treatment of cement-based materials and
  sequestration of CO<sub>2</sub>." *Journal of Hazardous Materials*, **112**(3), 193–205.
  <a href="http://dx.doi.org/10.1016/j.jhazmat.2004.04.019">http://dx.doi.org/10.1016/j.jhazmat.2004.04.019</a>
- Gunning, P. J., Hills, C. D., and Carey, P. J. (2009). "Production of lightweight
  aggregate from industrial waste and carbon dioxide." *Waste Management*, 29(10),
  2722–2728. < http://dx.doi.org/10.1016/j.wasman.2009.05.021.>
- 494 Gunning, P.J., Hills, C.D., Antemir, A., and Carey, P.J. (2011). "Secondary aggregate
- 495 from waste treated with carbon dioxide," Proc., Institute of Civil Engineers -
- 496 Construction Materials, 164(5), 231-239. < https://doi.org/10.1680/coma.1000011.>
- Hardin, B.O. (1985). "Crushing of soil particles." *Journal of Geotechnical Engineering*,
  111(10), 1177–1192.
- Klotz, E. U., and Coop, M. R. (2001). "An investigation of the effect of soil state on the
  capacity of driven piles in sands." *Geotechnique*, **51**(9), 733-751.
- Lade, P.V., Yamamuro, J.A., and Bopp, P.A. (1996). "Significance of particle crushing
  in granular materials." *Journal Geotechnical. Engineering American Society of Civil Engineers (ASCE)*, **122**(4), 309–316.< https://doi.org/10.1061/(ASCE)0733-</li>
  9410(1996)122:4(309) >
- Lake, C.B., Choi, H., Hills, C.D., Gunning, P. and Manaqibwala, I. (2016).
  "Manufactured Aggregate from Cement Kiln Dust." *Environmental Geotechnics,*

- 507 <http://dx.doi.org/10.1680/jenge.15.00074.>
- 508 Lee, K. L., and Farhoomand, I. (1967). "Compressibility and crushing of granular soil."
- 509 *Canadian Geotechnical Journal*, **4**(1), 68–86. <<u>http://dx.doi.org/10.1139/t67-012</u>.>
- 510 Marsal, R. J. (1967). "Large-scale testing of rockfills materials." J. Soil Mech. Found.
- 511 Engineering Division. American Society of Civil Engineers (ASCE), 93(2), 27–44.
- 512 Mun, W., and McCartney, J. S. (2017). "Effective Stress Analysis of the Undrained
- 513 Compression of Unsaturated Soils." 19th International Conference on Soil Mechanics
- 514 *and Geotechnical Engineering*. Seoul.
- 515 Shahnazari, H., and Rezvani, R. (2013). "Effective parameters for the particle breakage
- of calcareous sands: An experimental study." Engineering Geology, 159, 98-105.
- 517 <http://dx.doi.org/10.1016/j.enggeo.2013.03.005.>
- 518 Shipton, B., and Coop, M. R. (2012). "On the compression behaviour of reconstituted
- 519 soils." *Soil and Foundation*. **52**(4), 668-681.
- 520 <https://doi.org/10.1016/j.sandf.2012.07.008.>
- 521 Yamamuro, J. A., and Lade, P. V. (1993). "Effects of strain rate on instability of granular
- soils." *Geotechnical Testing Journal*, **16**(3), 304–313.
- 523 Zheng, W., and Tannant, D. (2016). "Frac sand crushing characteristics and morphology
- 524 changes under high compressive stress and implications for sand pack permeability."
- 525 Canadian Geotechnical Journal 53(9), 1412-1423. <a href="https://doi.org/10.1139/cgj-2016-">https://doi.org/10.1139/cgj-2016-</a>
- 526 0045.>
- 527 Zheghal, M. (2009). "The impact of grain crushing on road performance." Geotechnical

*and Geological Engineering.* **27**, 549–558.